3820 E. Broadway Ave. Spokane, WA 99202 Tel: 509.535.8841 Fax: 509.535.9589

# GEOTECHNICAL REPORT

To: Rudeen Development, LLC c/o Greg Jefferys

Cc: John Konen, Storhaug Engineering

Date: March 10, 2006

Re: Geotechnical Report, Evaluation of Drainage and Initial Characterization for Road Cuts, - Liberty

Lakeview Estates, Liberty Lake, WA (S06011)

# Scope and Project Description

We understand that you are proposing the construction of a new residential single-family hillside development on the west side of N Liberty Lake Road, near the intersection of N Liberty Lake Road and Settler Drive, as illustrated in the attached Site Map and Vicinity Map. The project site is approximately 23 acres in size.

Because of the relatively rocky conditions across the site, the project civil engineer, Storhaug Engineering, is developing drainage plans to make best use of subsurface conditions. We understand that the entrance may require a 35-foot thick cut to achieve desired grade.

You requested geotechnical explorations and analysis addressing drainage, stability assessment of proposed road cuts, and related earthwork. In order to expedite civil engineering design, this initial report only addresses subsurface infiltration potential for storm water, as well as initial characterization of road cuts.

#### **Field Explorations**

In order to identify subsurface conditions and to provide test sites for in-situ infiltration tests, we conducted 8 test pit explorations. The locations of the test pits are shown on the attached *Site Plan*. We completed the test pit excavations on the 26<sup>th</sup> of January 2006 using a Case 9010B trackhoe with 2.0-foot wide bucket. They ranged in depth from 2 to 19 feet. Additionally, 6 exploratory borings were completed at depths ranging from 20 to 30 feet below ground surface (bgs) using a Longyear 28 air rotary drill rig.

Test pit backfill was replaced with tamping of the track hoe bucket. Conditions encountered are described in the attached *Test Pit Logs*. A key, labeled *Guide to Soil and Rock Descriptions*, is also attached.

Two test pit infiltration tests were conducted in accordance with *Spokane County Guidelines for Stormwater Management*, Appendix I-4.3. One infiltration test was conducted in the vicinity of TP 1 and the other in the vicinity of TP4. In the following text they are referred to as the northern lobe (TP 1) and southern lobe (TP4). They were approximately located at the northeast corner and near the east central property line, near the existing housing development. These two locations were chosen for their position in topographic low areas and the likelihood that suitable material for drainage would be encountered with depth. Stabilized flow rates, head levels, and test pit dimensions are summarized for the two test sites on the attached table, *Test Pit Infiltration Data*.

## Setting

The ground surface slopes east at 15-50%, with the steepest terrain generally occurring on the eastern half of the site. The slopes above the site in the Legacy Hills development are more gradual at approximately 20% or less. According to a topographic plan provided by the project architect, the highest elevation on the site is in the southwest corner (approximately 2300 ft), about 225 feet higher than the lower portions on the eastern boundary near N Liberty Lake Rd. Based on the relatively thin organic layer at the surface, the site appears never to have been tilled. The surface is vegetated with a moderate growth of grasses, Ponderosa Pine, brush and numerous Precambrian metamorphic rock outcrops. To the south and east are an existing residential development and a city of Liberty Lake water tower. To the east is N Liberty Lake Rd, beyond which is a golf course and housing developments. To the north is a vacant field.

The geologic conditions on the subject site can generally be characterized by relatively thin overburden thickness (i.e., soil cover), relatively shallow depth to Precambrian metamorphic bedrock, and relatively shallow depth to groundwater on the eastern side of the site. However there can be significant lateral variability in subsurface conditions such as from the low-lying lobes underlain by gravel and upland terrain underlain at shallow depth by rock.

## Regional Geology

The majority of the site is composed of Hauser Lake Gneiss (Precambrian). This material is characterized by rusty weathering, medium-grained, well banded, foliated, and lineated mylonitic biotite-orthoclase-plagioclase-quartz gneiss, and schist that contains minor quartzite. Muscovite-biotite schist layers are less than 1 meter (m) thick and quartz-feldspar layers are more than 1 m thick. Bedding or foliation of schist zones are consistent across large areas of the site, but other discontinuities, such as joints and cleavage planes, are abundant, variably spaced, and variably oriented producing very complex masses of rock with respect to rock mechanics and groundwater flow.

The lobes of lower elevation, where the two infiltration tests were conducted, are composed of a different geologic group of flood deposited material from the Pleistocene time period. This material is a poorly sorted, stratified mixture of gravel, cobbles, boulders, and sand resulting from multiple episodes of catastrophic outbursts from glacier-dammed lakes, such as glacial Lake Missoula which inundated much of the present Clark Fork River drainage in Montana and Idaho.

# **Encountered Conditions**

Four distinct materials were encountered at the site with respect to properties relevant to development of the project: 1) SILT, 2) GRAVEL, 3) SILTY SAND, and 4) ROCK, as described further below. The first areas of the site explored were two small lobes of ground comprised primarily of flood deposited material, on the northeastern and southeastern portion of the site, where the land surface elevation is similar to that of N Liberty Lake Rd. Conditions encountered beneath these lobes included an approximately 1 to 2-foot thick layer of sandy SILT underlain by laminated GRAVEL with thin sand and silt laminations and lenses, to a depth exceeding 30 feet (this area includes TP 1, 2 & TB 9). Beneath the southeastern lobe of the site, at the base of the hillside, Precambrian metamorphic ROCK was encountered in TP 4 and 5, beneath the laminated GRAVEL layer. A few feet away from the hillside no rock was encountered in TB 11. Static groundwater levels in these areas ranged between 23 and 26 feet bgs. In TP 3 and TB 10, directly above and to the west of TP 1 & 2 SILTY SAND was encountered continuously to a depth of 25 feet bgs. The static ground water level in this area was recorded at approximately 22 feet bgs.

Beneath other locations explored at the site, a layer of loose to medium dense, sandy SILT was observed at the ground surface underlain by decomposing and competent Precambrian metamorphic ROCK. Depth to ROCK varies from 0.5 to 1 foot bgs in the borings that encountered this material. In general, the ROCK was composed of several feet of very decomposed and soft material, and then became moderately hard and less decomposed.

Below the sandy SILT unit are coarse-grained GRAVEL outburst flood deposits that include sand, cobbles, and boulders. The GRAVEL layer contains thin interbeds of silt and silty sand. The thickness of the flood materials ranged from 5 to greater than 29 feet, typically increasing to the east and northeast. The true thickness in several locations is not known because flood deposits were not completely penetrated. The conditions of the flood deposits appear to be medium dense to dense.

The grain size analyses show that the percentage passing the # 200 sieve in the minus 3/4 inch fraction of the flood deposits is 5 % or less. The permeability of this stratum is relatively consistent laterally but somewhat reduced vertically by finer textured laminations and lenses. The estimated permeability (k) of the GRAVEL layer ranges from 20 and 130 in/hr. The estimated k-value was calculated from results of field infiltration tests, as described further in the next section.

At five test pit locations, TP 4 through TP 8, ROCK caused refusal below the SILT and GRAVEL at depths ranging from 2 to 11 feet.

During the exploration, groundwater was encountered at three locations but only one (TB 10) was, above the low-lying eastern boundary of the site. At wetter times of the year or during wetter climatic cycles, groundwater could be found perched on the ROCK at other locations. A stream, likely intermittent, was observed at the north end of the project near the area of the thickest proposed cut. The stream is in the vicinity of TP 3 and TB 10, immediately north, and flows east towards TP 1 and N Liberty Lake Road.

#### Conclusions and Recommendations

The majority of the subject site is unsuitable for infiltration; however, the two lobes on the eastern side of the site appear to be hydraulically connected to flood deposits and are capable of receiving sizable volumes of water over an extended period of time. Field infiltration tests and correlation with laboratory determined index properties demonstrate that the permeability of the flood deposits qualify the GRAVEL layer as a permeable target layer for infiltration.

Soil permeability was assessed by visual observations of soil in borings and test pits, laboratory testing of grain size distribution, and field infiltration testing in two test pits. The stabilized flow rates in these tests were 40 and 260 gallons per minute (gpm) in TP 1 and 2, respectively. Drywell infiltration rates were determined in accordance with the *Infiltration Rates and Soil Classification Correlation (IRSCC)*, *May 28*, 2004 developed by Spokane County and the City of Spokane in conjunction with Budinger & Associates, Inc., Cummings Geotechnology, Inc., and GeoEngineers. Criteria include safety factors of 1.8 and 1.3 for TP 1 and 2, respectively, resulting in maximum design infiltration rates of 0.2 and 1.5 cfs, respectively for Type B (two barrel deep) drywells.

We recommend installing three-barrel deep drywells at the northern site to expose as much surface area of the permeable soil as possible and two-barrel deep drywells in the southern site based upon silty laminations and lenses as well as bedrock and groundwater boundaries. Furthermore, we recommend a maximum design outflow rate per drywell of 0.13 cubic feet per second (cfs) beneath the northern drainage area and 0.80 cfs beneath the southern area. The maximum total outflow of the drainage areas should not exceed 0.80 cfs beneath the northern drainage area or 3.2 cfs beneath the southern area, which would limit the total number of drywells in each area to 6 and 4, respectively.

We recommend a minimum center-to-center drywell spacing of 40 feet and a minimum of a 4-foot separation from the base of the drywells to bedrock or static groundwater levels.

Groundwater in the area of TB 10 may have significant impact to road cut design as the depth to groundwater was approximately 22 feet bgs, while current plans are to cut the road to a depth of 35 feet bgs.

#### Limitations

Services were limited to the exploration, testing, and analysis described herein. This report should not be used for other purposes. Geotechnical engineering for other civil, environmental, or permitting aspects of the project are beyond the scope of this involvement. Other limitations are summarized in the attached document entitled *Important Information About Your Geotechnical Engineering Report*.

We appreciate the opportunity to offer this service. Please call if you have any questions.

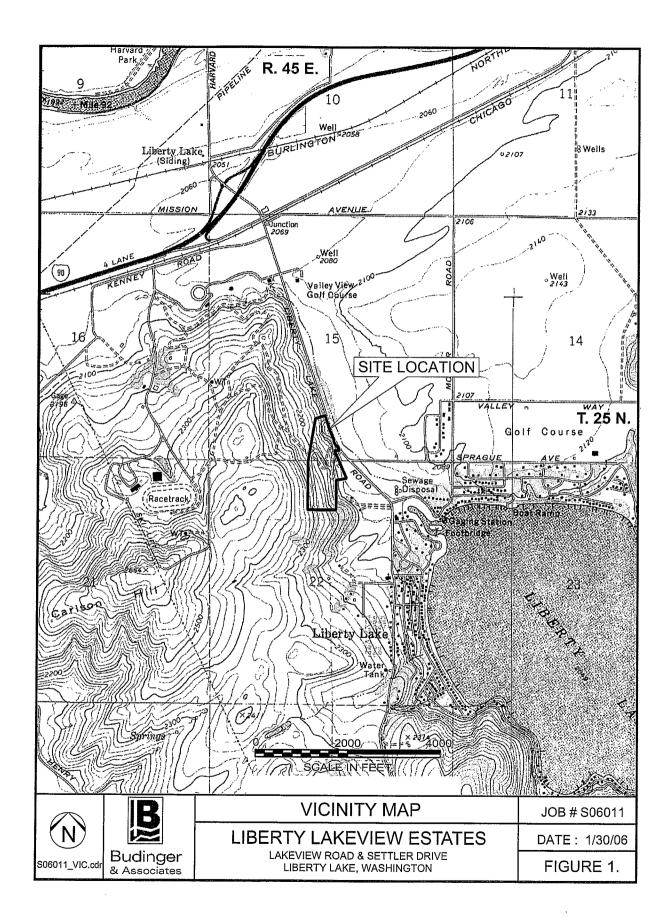
Respectfully Submitted: BUDINGER & ASSOCIATES, INC.

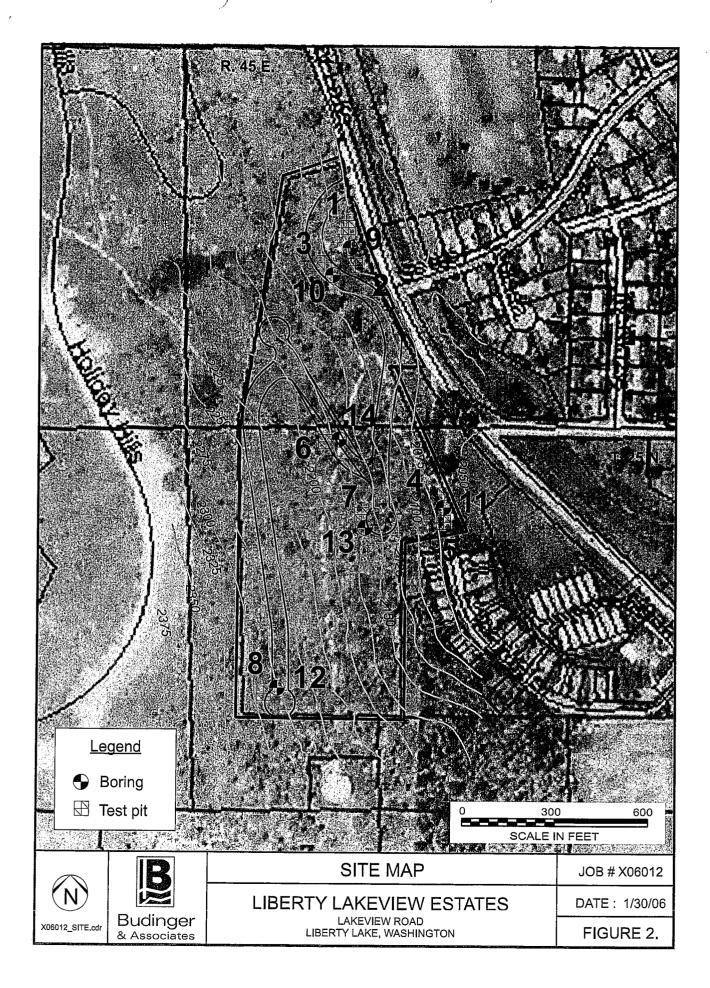
Ryan Molsee Hydrogeologist

John E. Finnegan, PE Geotechnical Engineer, Principal

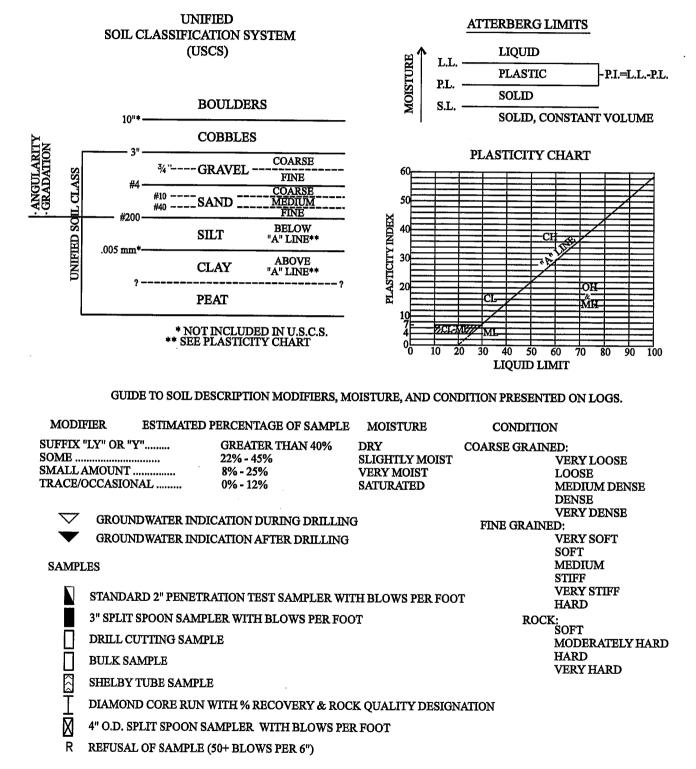
Addressee – 6 Attachments

- Laboratory Summary
- Vicinity Map, Figure 1
- Site Map, Figure 2
- Guide to Soil & Rock Descriptions, Figure 3
- Test Pit Logs, Figures 4-1 thru 4-12
- Test Pit Infiltration Data, Figures 5-1 and 5-2
- Grain Size Distribution Results, Figure 6
- Important Information About Your Geotechnical Engineering Report





# GUIDE TO SOIL & ROCK DESCRIPTIONS



Budinger & Associates, Inc. Geotechnical & Environmental Engineers Construction Materials Testing & Inspection

Date:

1-26-06

Excavator:

Budinger & Assoc., Inc.

Equipment: Location:

Case 9010B track hoe, 24" bucket .NE side

Surface:

grass and weeds

Logged by: R. Molsee

о DEРТН		MOISTURE, COLOR, CONDITION		SCRIPTION	SOIL LOG	·		
		moist, dark brown, medium dense	SANDY SILT				1	
5		slightly moist, light brown, medium dense to dense	GRAVEL, some Sa sub-rounded, medit	nd, occasional Boulders um	s, 0000			
	i							
10			laminated with thin and sand	layers and lenses of silt				
15	·							·
	1	no free groundwater	End of Ev	cavation @ 19 ft				
20		observed		ouvalion @ 10 li				
25 								
30					,			
35							FIGURE	
	B	Budinger & Associates 3820 E. Broadway Ave Spokane, WA 99202	- 5	TEST PIT L Project: Liberty L Location: N. Libe	Lakeview		FIGUR  County, WA	., .

Date:

1-26-06

Excavator:

Equipment:

Budinger & Assoc., Inc. Case 9010B track hoe, 24" bucket

Location:

NE side

Surface:

grass and weeds

Logged by: R. Molsee

DEPTH	MOISTURE, COLOR, CONDITION	DESCRIPTION	SOIL LOG	
_0	alightly moist dayle	CANDYOUT	12 17 17	
	brown, medium dense	SANDY SILT	<b>- 64</b> 7	<u>.</u>
	dry to slightly moist,	GRAVEL, some Sand, occasional Boulders, sub-rounded, medium	19 19	
	slightly moist, dark brown, medium dense dry to slightly moist, light brown, medium dense to dense		[6]	
			609	
5			19,12	•
	·		Path	
			623	
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10		<u>.</u>	P 2 9	
		laminated with thin layers and lenses of silt and sand	814	•
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			10 Pol	
			[8]	
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13			19,61	•
<b> </b>			1976	•
			PLAS	
·····			122	
	no free groundwater	End of Excavation @ 19 ft	<u>-₽₩</u> 4	
20	no free groundwater observed	End of Excavation @ 15 ft		
				•
<b></b>				
25				
				;
30				
	[			
35				



**TEST PIT LOGS** 

FIGURE 4-2

Project: Liberty Lakeview Estates

Location: N. Liberty Lake Rd, Spokane County, WA

Number: S06011

TPWNE S06011.GPJ BUDINGER.GDT 3/10/06

Date:

1-26-06

Excavator:

Budinger & Assoc., Inc.

Equipment:

Case 9010B track hoe, 24" bucket

Location:

N side

Surface:

grass and weeds

Logged by: R. Molsee

	· · · · · · · · · · · · · · · · · · ·			
<b>=</b>	MOISTURE, COLOR, CONDITION		စ္က	
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0	≥ 0		05	
	moist, dark brown.	SILT	<del>  , , , ,  </del>	
	moist, dark brown, loose to medium dense / moist, light brown, medium dense to dense	SAND, some Gravel, fine to medium		
	moist, light brown, medium dense to dense	· ·		
5				
				•
10				
	moist, light gray, medium dense to dense	SAND, some Gravel and Cobbles, fine to medium	- 0.01	
	medium dense to dense	medium	6 1	
15				
10 1	no free groundwater observed	End of Excavation @ 15 ft	• : : : •	
	observed			
		·		,
20				
25_				
30				
!				
,,,,				



TPWNE S06011.GPJ BUDINGER.GDT 3/10/06

Budinger & Associates

3820 E. Broadway Ave. Spokane, WA 99202 **TEST PIT LOGS** 

FIGURE 4-3

Project: Liberty Lakeview Estates

Location: N. Liberty Lake Rd, Spokane County, WA

Date:

1-26-06

Excavator:

Equipment:

Budinger & Assoc., Inc. Case 9010B track hoe, 24" bucket

Location:

E Central

Surface:

grass and weeds

Logged by: R. Molsee

	· · · · · · · · · · · · · · · · · · ·				
о DEPTH		MOISTURE, COLOR, CONDITION	DESCRIPTION	SOIL LOG	
		moist, dark brown, loose to medium dense	SILT		
5		dry to slightly moist, gray with brown, medium dense to dense	SAND, some Gravel and Silt  Some laminated gravel with thin layers of silt		
10		slightly moist, brown to orange, loose to medium dense dry, gray, soft	SAND, medium to coarse  GNEISS/SCHIST		
15		no free groundwater observed	End of Excavation @ 11 ft		
20			·		
25		•			
30					
35					
35					

3820 E. Broadway Ave. Spokane, WA 99202

**TEST PIT LOGS** 

FIGURE 4-4

Project: Liberty Lakeview Estates

Location: N. Liberty Lake Rd, Spokane County, WA

Number: S06011

TPWNE S06011,GPJ BUDINGER,GDT 3/10/06

Date:

1-26-06

Excavator:

Budinger & Assoc., Inc.

Equipment:

Case 9010B track hoe, 24" bucket

Location:

E Central

Surface:

grass and weeds

Logged by: R. Moisee

о ОЕРТН		MOISTURE, COLOR, CONDITION	DE	SCRIPTION	SOIL LOG	
0		moist to wet, dark brown, loose to medium dense moist, light brown, medium dense	SILT SILTY SAND, medi	um to coarse		
5	1	dry, gray, soft no free groundwater	Weathered GNEISS	S/SCHIST xcavation @ 6 ft		·
10		observed		ASSECTION & OIL		
20						
30						
	B	Budinger & Associates 3820 E. Broadway Ave Spokane, WA 99202	<b>-</b> 5	TEST PIT LO Project: Liberty La Location: N. Libert Number: S06011	akeview Estate	FIGURE 4-5 es pokane County, WA

# **TEST PIT 6** Date: 1-26-06 Excavator: Budinger & Assoc., Inc. Logged by: R. Moisee Equipment: Case 9010B track hoe, 24" bucket Location: , Central Surface: grass and weeds SOIL LOG DEPTH DESCRIPTION moist, dark brown, loose to medium dense SILTY SAND and weathered gneiss/schist dry to slightly moist, (transition zone) light brownish gray, GNEISS/SCHIST very dense dry to slightly moist, gray, soft End of Excavation @ 2 ft 5 no free groundwater observed 10 15 20 25 30 TPWNE S06011.GPJ BUDINGER.GDT 35 FIGURE 4-6 TEST PIT LOGS Budinger & Associates Project: Liberty Lakeview Estates

3820 E. Broadway Ave. Spokane, WA 99202

Location: N. Liberty Lake Rd, Spokane County, WA

Date:

1-26-06

**Excavator:** 

Equipment: Location:

Budinger & Assoc., Inc. Case 9010B track hoe, 24" bucket

Central

Surface:

grass and weeds

Logged by: R. Molsee

DEPT		MOISTU	DESCRIPTION	SOIL L(	
0		moiat daile brown	CUT	<del></del>	
		moist, dark brown, loose to medium dense dry to moist, grayish brown, soft no free groundwater observed	SILT Weathered GNEISS/SCHIST with sand		
		brown, soft	·	_\\\\	{
		no free groundwater observed	End of Excavation @ 2.5 ft		
5					
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3820 E. Broadway Ave. Spokane, WA 99202

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35

**TEST PIT LOGS** 

FIGURE 4-7

Project: Liberty Lakeview Estates

Location: N. Liberty Lake Rd, Spokane County, WA

Date:

1-26-06

**Excavator:** 

Equipment:

Budinger & Assoc., Inc. Case 9010B track hoe, 24" bucket

Location: Surface:

,SW corner grass and weeds Logged by: R. Moisee

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	m, Z				
ОЕРТН	MOISTURE, COLOR, CONDITION	DES	CRIPTION	SOIL LOG	
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0	2 0			"	
	moist, dark brown,	SILT			
	\loose to medium dense / moist, grayish brown,	SILTY SAND, with w	eathered rock		
	dense dry to slightly moist,	GNEISS/SCHIST with	h sama sand		
	\gray, soft /	GNEISS/SCHIST with	cavation @ 3 ft		
5	no free groundwater observed		,		
	observed				
10					
15	₹				
	·				
20					
		,			
25					
30					
,,					
35					
	Rudingar	-	TEST PIT L	OGS	FIGURE 4-8
	Budinger & Associates	Ī	Project: Liberty L	.akeview Es	tates
	& Associates	6			
			Location: N. Liha	けいしつじつ ロイ	i Chakana Causti 1878
	3820 E. Broadway Ave Spokane, WA 99202		Location: N. Libe Number: S06011		I, Spokane County, WA



Date of Boring:2-8-06

Driller: Budinger & Assoc., Inc.
Type of Drill: Longyear 28
Location: NE side

Surface:

grass and weeds

**Logged by:** R. Molsee **Size of hole:** 4-1/2" O.D. air rotary

				<del></del>			TE	ST RES	ULTS		
SAMPLES RQD, BLOW	COUNTS N (% RECOVERY) (blows/6" (italics))	MOISTURE, COLOR, CONDITION	DE.	SCRIPTION	SOIL LOG	WATER CON STANDARD 3" SPLIT SPO	PL <b> </b> NTENT () PEN TEST, DON PENE	N-VALUE TRATION,	(OBSER , BLOWS/	/FT	<b>□</b>
		moist, dark brown,	SILT			10 20	30 40	50	60 70	80	90
		loose to medium dense / slightly moist, light brown, medium dense	GRAVELLY SAND, amount-some Cobb subangular-subroun	les, occasional Boulder,	° ()						
5											
10			laminations and lens	es of silt							
15	(30%)					\$1					
20 F	R (0%)		material. After borin	oserved in SANDY SILT g was complete,							+
25		7	Water was initially ob- material. After boring groundwater level ros feet bgs.	se to approximately 20							
		saturated, light brown,	SANDY SILT					İ			
30	(22%)	\medium dense to dense/ saturated, light brown, dense	GRAVELLY SAND, s amount-some Cobbl subangular-subround	es, occasional Boulder,							
<b>\</b>	\ <u>~~</u> /0)				° 0						
			End of E	Soring @ 30 ft							
35				BORING LO	GS		F	FIGL	JRE	4-6	⊥ <b>)</b>
	B	Budinger & Associates 3820 E. Broadway Av	S	Project: Liberty Lak Location: N. Liberty	eview						
L	$\cong$	Spokane, WA 99202	<b>-</b> .	Number: S06011		, ,		-31	-		



Date of Boring:2-14-06

Driller: Driller: Budinger & Assoc., Inc.
Type of Drill: Longyear 28

Location: Northern

Surface:

grass and weeds

**Logged by:** R. Molsee **Size of hole:** 4-1/2" O.D. air rotary

DEPTH		COUNTS N RECOVERY)	(blows/6" (italics))	MOISTURE, COLOR, CONDITION	DE	SCRIPTION	SOIL LOG			3 LIMITS	L	NEO	ULTS	<b></b>  LL	
o DE	SAN	% 2005 7005 7005	9/swolq)	MOIS CO CONI			SOIL	3" SF	LIT SP	PEN TE	NETR	ATION,	BLOWS	S/FT	
				moist, dark brown,	SILT			10	20	30	40	<u>50 ε</u>	1	80	90
				\loose / moist, brown/red brown,	SILTY SAND, mediu approximately 20 fe	m to coarse (saturated at									
				very dense	approximately 20 to	,									
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10										-	-	<b> </b>			_
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			Ì		measured at 22' bgs										
				7	7_										
25		R (3	0%)						-		+-	-			+10
			-	-	End of E	oring @ 25 ft									
••••															
30															
															İ
35															
	I	شر ر	_	Rudinger	~	BORING LOC	S			W	FI	GU	RE	4-	<u> 10</u>
		-		Budinger & Associates	•	Project: Liberty Lake	eview	Esta	ates						
				3820 E. Broadway Ave		Location: N. Liberty	Lake	Rd.	Spol	kane	Соц	ntv.	WA		
				Spokane, WA 99202	<b>5.</b>	l		,	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			,	·		



Date of Boring:2-15-06

Driller: Budinger & Assoc., Inc.
Type of Drill: Longyear 28

Location: Surface:

, E Central

grass and weeds

**Logged by:** R. Molsee **Size of hole:** 4-1/2" O.D. air rotary

		graes and weeds					TE	ST RES	HTC		
SAMPLES RQD, BLOW COUNTS N	(% RECOVERY) (blows/6" (italics))	MOISTURE, COLOR, CONDITION	DE	SCRIPTION	SOIL LOG	ATTERBER WATER CO STANDARE 3" SPLIT SF	G LIMITS PL   NTENT O PEN TEST	, N-VALUE	(OBSERV BLOWS/F	Т	<b>■</b>
		moist, brown, loose to medium dense	SILT, occasional Gr roots, poorly graded	avel, trace Organics: (fine-medium)		10 20	30 40	30 6	70	80 8	Ĭ
10		dry to slightly moist, grayish brown, very dense	GRAVELLY SAND, Cobbles, occasiona subangular-subroun	Boulder, ded							
	(16%)										+100
25 R	(4%)	saturated <u>5</u>	no saturated cutting boring, however afte was detected at app	s were observed during r completion water level orximately 26' bgs							+100
			End of I								
35	3	Budinge & Associate 3820 E. Broadway Av Spokane, WA 99202	S re.	Project: Liberty Lake Location: N. Liberty Number: S06011	eview			FIGU		4-1	1

Date of Boring:2-15-06

Driller: Budinger & Assoc., Inc.

Type of Drill: Longyear 28 Location: SW corner Surface: grass and weeds Logged by: R. Molsee

Size of hole: 4-1/2" O.D. air rotary

TEST RESULTS RQD, BLOW COUNTS N (% RECOVERY) blows/6" (italics) ATTERBERG LIMITS L0G SAMPLES DEPTH WATER CONTENT O DESCRIPTION STANDARD PEN TEST, N-VALUE (OBSERVED) 3" SPLIT SPOON PENETRATION, BLOWS/FT SILTY SAND, occasional Gravel, trace moist, brown, loose to medium dense Organics (roots), poorly graded (fine-medium) dry, gray, soft decomposed GNEISS/SCHIST 5 10 15 20 moderately hard becomes moderatly hard at approximatly 22' 25 no free groundwater observed End of Boring @ 25 ft 30 BUDINGER, GDT LWWWT NO ELEV S06011.GPJ 35 **FIGURE 4-12 BORING LOGS** 

Budinger 3820 E. Broadway Ave. Spokane, WA 99202

Project: Liberty Lakeview Estates

Location: N. Liberty Lake Rd, Spokane County, WA

Date of Boring:2-16-06

Driller: Budinger & A
Type of Drill: Longyear 28
Location: Central

Budinger & Assoc., Inc.

Surface:

grass and weeds

**Logged by:** R. Molsee **Size of hole:** 4-1/2" O.D. air rotary

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DEPTH	SAMPLES RQD, BLOW COUNTS N (% RECOVERY blows/6" (italics,	MOISTURE, COLOR, CONDITION	DE:	SCRIPTION	SOIL LOG	STAND	R CONTI PARD PE	N TES	Т, N-V.	ALUE	(OBSE	LL :RVED) S/FT	. 5
0	8 8					10	_20	30 4	40 5	i0 6	0 7	0 80	90
		slightly moist, brown, loose to medium dense dry, gray with brown, soft	SILTY SAND, occas Organics (roots), po decomposed GNEIS	orly graded (fine-medium) /									
5													
						0			İ				
		,											
10		moderately hard	GNEISS/SCHIST be of approximately 9' b	comes harder at a depth gs									
15													
20	3												
		,					1						
25		no free groundwater observed	End of E	oring @ 25 ft									
30													
35				BORING LOG	SS			<u> </u>	Fic	GU	RF	<u> </u> [ 4-	.1:
	B	Budinger & Associates	r s	Project: Liberty Lake	view					•		<b>- T</b>	
		3820 E. Broadway Ave Spokane, WA 99202		Location: N. Liberty I Number: S06011	_ake	Kd, S	poka	ne C	Cour	nty, \	WΑ		

Date of Boring:2-17-06

Driller: Budinger & Assoc., Inc. Longyear 28

Location:

, Central

Surface: grass and weeds

**Logged by:** R. Molsee **Size of hole:** 4-1/2" O.D. air rotary

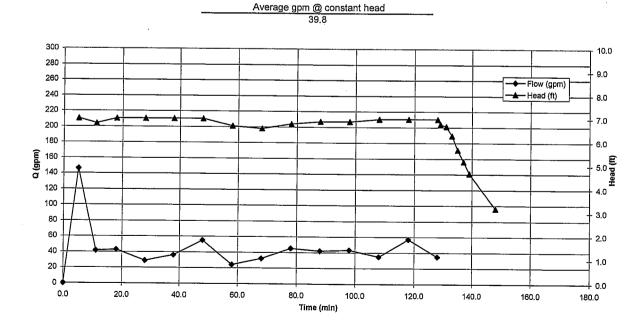
о ДЕРТН	SAMPLES RQD, BLOW COUNTS N (% RECOVERY) (blows/6" (italics))	MOISTURE, COLOR, CONDITION	DE	SCRIPTION	SOIL LOG	WAT STAI 3" SI	PLIT SP	NTEI PEN NOON	MITS NT C NTES	) ſ, N-VA ETRAT	LUE	(OBSE	LL ERVEC /S/FT	))   
5		dry, light gray, soft	weathered GNEISS	SCHIST		1	0 20	3	0 4	0 50	) 6	0 7	0 8	0 9
10						0								
15		moist, reddish brown, medium dense dry, light gray, moderately hard	SANDY SILT with or GNEISS/SCHIST	ganics and tree roots										
20														
25		no free groundwater observed	End of E	oring @ 20 ft							:			
30														
35				BORING LC	)GS					FIG	) LJ	RF	Ξ 4	-14
	B	Budinge & Associates 3820 E. Broadway Av Spokane, WA 99202		Project: Liberty La Location: N. Libert Number: S06011	keview			kar			-			•



Figure 5-1
Test Pit Infiltration Data

WL BGS = water level depth below ground surface (ft)
WL Elev = water level elevation (ft)

Test Pit #1					<u> </u>		
Total Depth (ft) Surface Elevation (ft) Bottom Elevation (ft)					10.0 2330		<b>1</b>
Bottom dimensions Gravel		:			2320 3' x 7' 3' to 10' below	∕ grade₋	
			Total flo	ow rate			
Date/Time	Time (min)	meter 1 (gal)	cum. (gal)	(gpm)	WL BGS	WL Elev	Head
1/31/2006 14:52	0.00	272900	0	0			
1/31/2006 14:57	5.00	273630	730	146.0	3.0	2327.0	7.0
1/31/2006 15:03	11.00	273880	980	41.7	3.2	2326.8	6.8
1/31/2006 15:10	18.00	274180	1280	42.9	3.0	2327.0	7.0
1/31/2006 15:20	28.00	274470	1570	29.0	3.0	2327.0	7.0
1/31/2006 15:30	38.00	274830	1930	36.0	3.0	2327.0	7.0
1/31/2006 15:40	48.00	275380	2480	55.0	3.0	2327.0	7.0
1/31/2006 15:50	58.00	275620	2720	24.0	3.3	2326.7	6.7
1/31/2006 16:00	68.00	275940	3040	32.0	3.4	2326.6	6.6
1/31/2006 16:10	78.00	276390	3490	45.0	3.2	2326.8	6.8
1/31/2006 16:20	88.00	276810	3910	42.0	3.1	2326.9	6.9
1/31/2006 16:30	98.00	277240	4340	43.0	3.1	2326.9	6.9
1/31/2006 16:40	108.00	277590	4690	35.0	3.0	2327.0	7.0
1/31/2006 16:50	118.00	278160	5260	57.0	3.0	2327.0	7.0
1/31/2006 17:00	128.00	278510	5610	35.0	3.0	2327.0	7.0
1/31/2006 17:01	129.00				3.2	2326.8	6.8
1/31/2006 17:03	131.00				3.3	2326.7	6.7
1/31/2006 17:05	133.00				3.7	2326.3	6.3
1/31/2006 17:07	135.00				4.3	2325.7	5.7
1/31/2006 17:09	137.00				4.8	2325.2	5.2
1/31/2006 17:11	139.00				5.3	2324.7	4.7
1/31/2006 17:20	148.00				6.8	2323.2	3.2

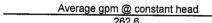


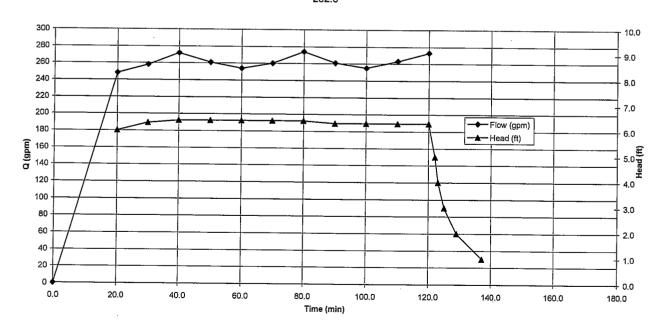
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Figure 5-2
Test Pit Infiltration Data

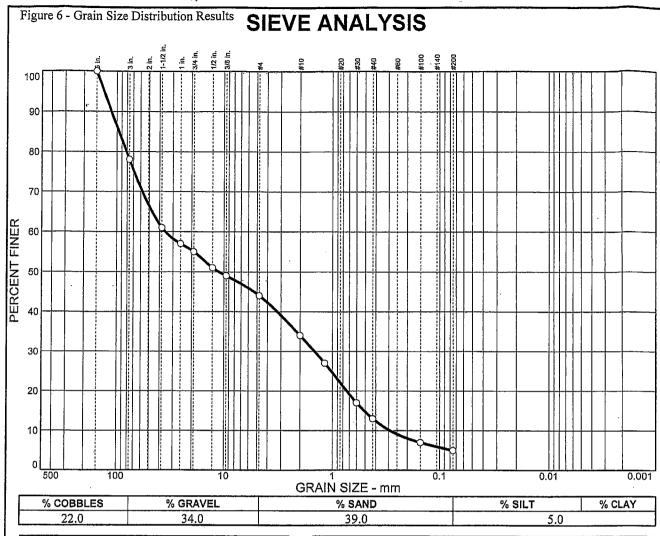
WL BGS = water level depth below ground surface (ft)
WL Elev = water level elevation (ft)

Test Pit #2							
Total Depth (ft) Surface Elevation (ft) Bottom Elevation (ft)					12.0 2330 2318		1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1
Bottom dimensions Gravel				i	3' x 7'		
014101		<del></del>	Total flo	w rate	5' to 12' below	grade	
Date/Time	Time (min)	meter 1 (gal)	cum. (gal)	(gpm)	WL BGS	WL Elev	Head
2/2/2006 10:20	0.00	279510	0	(92.17)	WE BOO	VVL LICV	Head
2/2/2006 10:40	20.00	284470	4960	248.0	6.0	2324.0	6.0
2/2/2006 10:50	30.00	287050	7540	258.0	5.7	2324.3	6.3
2/2/2006 11:00	40.00	289770	10260	272.0	5.6	2324.4	6.4
2/2/2006 11:10	50.00	292380	12870	261.0	5.6	2324.4	6.4
2/2/2006 11:20	60.00	294920	15410	254.0	5.6	2324.4	6.4
2/2/2006 11:30	70.00	297520	18010	260.0	5.6	2324.4	6.4
2/2/2006 11:40	80.00	300260	20750	274.0	5.6	2324.4	6.4
2/2/2006 11:50	90.00	302870	23360	261.0	5.7	2324.3	6.3
2/2/2006 12:00	100.00	305420	25910	255.0	5.7	2324.3	6.3
2/2/2006 12:10	110.00	308050	28540	263.0	5.7	2324.3	6.3
2/2/2006 12:20	120.00	310780	31270	273.0	5.7	2324.3	6.3
2/2/2006 12:22	122.00				7.0	2323.0	5.0
2/2/2006 12:23	123.00				8.0	2322.0	4.0
2/2/2006 12:25	125.00			77.11.12.1	9.0	2321.0	3.0
2/2/2006 12:29	129.00				10.0	2320.0	2.0
2/2/2006 12:37	137.00				11.0	2319.0	1.0





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SIEVE	PERCENT	SPEC.*	PASS?
SIZE	FINER	PERCENT	(X=NO)
6 in. 3 in. 1.5 in. 1 in. 3/4 in. 1/2 in. 3/8 in. #10 #10 #40 #200	100.0 78.0 61.0 57.0 55.0 51.0 49.0 44.0 34.0 27.0 17.0 13.0 7.0 5.0		

	Soil Description	
PL=	Atterberg Limits	Pl≃
	Coefficients	
D <sub>85</sub> = 95.6 D <sub>30</sub> = 1.47 C <sub>u</sub> = 121.54	$D_{60}$ = 35.5 $D_{15}$ = 0.512 $C_{c}$ = 0.21	D <sub>50</sub> = 11.1 D <sub>10</sub> = 0.292
USCS= SP-SM	Classification AASHT	<b>)=</b> .
Sampled by B&A	Remarks	

(no specification provided)

Sample No.: 1 (06-0064)

Source of Sample: On site

Date: 1/27/06 Elev./Depth:

Location:

Client:

Project: Liberty Lake Estates

& ASSOCIATES, INC.

**BUDINGER** 

Project No: S06011

Reviewed By:

